

Ramp/Weave Analysis

- 2012 Existing
- 2035 No Build
- 2035 Build Alternatives



VHB, Inc
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Merge Analysis

Analyst: NBS (Ex7_SB-On_AM)
Agency/Co.: VHB
Date performed: 7/3/2012
Analysis time period: AM Peak Hour
Freeway/Dir of Travel: I-293 / SB
Junction: Exit 7 SB On Ramp
Jurisdiction: NHDOT
Analysis Year: 2012 Existing Conditions
Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	2	
Free-flow speed on freeway	60.0	mph
Volume on freeway	1830	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	655	vph
Length of first accel/decel lane	600	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1830	655		vph
Peak-hour factor, PHF	0.89	0.85		
Peak 15-min volume, v15	514	193		v
Trucks and buses	4	2		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.980	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2097	778	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
EQ
P = 1.000 Using Equation 0
FM
 $v_{12} = v_F(P_{FM}) = 2097 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v	2875	4600	No
FO			
v	2875	4600	No
R12			

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 23.8 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable,	M = 0.348	
	S	
Space mean speed in ramp influence area,	S = 53.7	mph
	R	
Space mean speed in outer lanes,	S = N/A	mph
	0	
Space mean speed for all vehicles,	S = 53.7	mph

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Merge Analysis

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 Agency/Co.: VHB
 Date performed: 7/3/2012
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 Freeway/Dir of Travel: I-293 / SB
 Junction: Exit 6 SB On Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2012 Existing Conditions
 Description: FEET I-293

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	2		
Free-flow speed on freeway	60.0	mph	
Volume on freeway	2550	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	385	vph	
Length of first accel/decel lane	300	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	655	vph	
Position of adjacent Ramp	Upstream		
Type of adjacent Ramp	Off		
Distance to adjacent Ramp	1500	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2550	385	655	vph
Peak-hour factor, PHF	0.89	0.93	0.80	
Peak 15-min volume, v15	716	103	205	v
Trucks and buses	4	2	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.980	0.990	1.000	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2922	418	819	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
EQ
P = 1.000 Using Equation 0
FM
 $v_{12} = v_F (P_{FM}) = 2922 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v	3340	4600	No
FO			
v	3340	4600	No
R12			

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 29.5 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence D

Speed Estimation

Intermediate speed variable,	M = 0.410	
	S	
Space mean speed in ramp influence area,	S = 52.6	mph
	R	
Space mean speed in outer lanes,	S = N/A	mph
	O	
Space mean speed for all vehicles,	S = 52.6	mph

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Diverge Analysis

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 Date performed: 7/3/2012
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 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 6 NB Off Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2012 Existing Condition
 Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	60.0	mph
Volume on freeway	1825	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	45.0	mph
Volume on ramp	955	vph
Length of first accel/decel lane	275	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1825	955		vph
Peak-hour factor, PHF	0.91	0.85		
Peak 15-min volume, v15	501	281		v
Trucks and buses	5	1		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.976	0.995	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2056	1129	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 1.000 \quad \text{Using Equation 0}$$

$$FD$$

$$v_{12} = v_R + (v_F - v_R) P_{FD} = 2056 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2056	4600	No
v_{12}	2056	4400	No
$v_{FO} = v_F - v_R$	927	4600	No
v_R	1129	2100	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 19.5 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	$D = 0.400$	
	S	
Space mean speed in ramp influence area,	$S = 52.8$	mph
	R	
Space mean speed in outer lanes,	$S = N/A$	mph
	0	
Space mean speed for all vehicles,	$S = 52.8$	mph

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Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	2	
Free-flow speed on freeway	60.0	mph
Volume on freeway	870	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	40.0	mph
Volume on ramp	610	vph
Length of first accel/decel lane	250	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	870	610		vph
Peak-hour factor, PHF	0.91	0.92		
Peak 15-min volume, v15	239	166		v
Trucks and buses	5	2		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.976	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	980	670	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
EQ
P = 1.000 Using Equation 0
FM
 $v_{12} = v_{F} (P_{FM}) = 980 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v	1650	4600	No
FO			
v	1650	4600	No
R12			

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 16.5 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	M = 0.321	
	S	
Space mean speed in ramp influence area,	S = 54.2	mph
	R	
Space mean speed in outer lanes,	S = N/A	mph
	O	
Space mean speed for all vehicles,	S = 54.2	mph

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Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	60.0	mph
Volume on freeway	1480	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	45.0	mph
Volume on ramp	545	vph
Length of first accel/decel lane	325	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1480	545		vph
Peak-hour factor, PHF	0.91	0.68		
Peak 15-min volume, v15	407	200		v
Trucks and buses	5	2		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5		1.5	
Recreational vehicle PCE, ER	1.2		1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	1667	809	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 1.000 \quad \text{Using Equation 0}$$

$$v_{12} = v_R + (v_F - v_R) \frac{P}{FD} = 1667 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	1667	4600	No
v_{12}	1667	4400	No
$v_{FO} = v_F - v_R$	858	4600	No
v_R	809	2100	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 15.7 \quad \text{pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	$D = 0.371$	
Space mean speed in ramp influence area,	$S_R = 53.3$	mph
Space mean speed in outer lanes,	$S_0 = \text{N/A}$	mph
Space mean speed for all vehicles,	$S = 53.3$	mph

HCS2000: Ramps and Ramp Junctions Release 4.1f

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Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	2	
Free-flow speed on freeway	60.0	mph
Volume on freeway	1585	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	390	vph
Length of first accel/decel lane	600	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp
Volume, V (vph)	1585	390	vph
Peak-hour factor, PHF	0.92	0.86	
Peak 15-min volume, v15	431	113	v
Trucks and buses	5	2	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	1766	458	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
EQ
P = 1.000 Using Equation 0
FM
 $v_{12} = v_F (P_{FM}) = 1766 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v	2224	4600	No
FO			
v	2224	4600	No
R12			

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 18.8 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	M = 0.315	
	S	
Space mean speed in ramp influence area,	S = 54.3	mph
	R	
Space mean speed in outer lanes,	S = N/A	mph
	O	
Space mean speed for all vehicles,	S = 54.3	mph

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Merge Analysis

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 Jurisdiction: NHDOT
 Analysis Year: 2012 Existing Conditions
 Description: FEET I-293

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	2		
Free-flow speed on freeway	60.0	mph	
Volume on freeway	2075	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	335	vph	
Length of first accel/decel lane	300	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	405	vph	
Position of adjacent Ramp	Upstream		
Type of adjacent Ramp	Off		
Distance to adjacent Ramp	1500	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2075	335	405	vph
Peak-hour factor, PHF	0.92	0.96	0.87	
Peak 15-min volume, v15	564	87	116	v
Trucks and buses	5	1	1	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.995	0.995	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2312	351	468	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
EQ
P = 1.000 Using Equation 0
FM
 $v_{12} = v_F (P_{FM}) = 2312 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v	2663	4600	No
FO			
v	2663	4600	No
R12			

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 24.2 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable,	M = 0.356	
Space mean speed in ramp influence area,	S _R = 53.6	mph
Space mean speed in outer lanes,	S ₀ = N/A	mph
Space mean speed for all vehicles,	S ₀ = 53.6	mph

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Diverge Analysis

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 Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	60.0	mph
Volume on freeway	3310	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	45.0	mph
Volume on ramp	1070	vph
Length of first accel/decel lane	275	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3310	1070		vph
Peak-hour factor, PHF	0.91	0.94		
Peak 15-min volume, v15	909	285		v
Trucks and buses	5	1		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.976	0.995	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3728	1144	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)
EQ
P = 1.000 Using Equation 0
FD
 $v_{12} = v_R + (v_F - v_R) P = 3728$ pc/h
FD

Capacity Checks

	Actual	Maximum	LOS F?
$v_{12} = v_{Fi}$	3728	4600	No
v_{12}	3728	4400	No
$v_{FO} = v_F - v_R$	2584	4600	No
v_R	1144	2100	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 33.8$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence D

Speed Estimation

Intermediate speed variable,	D = 0.401	
Space mean speed in ramp influence area,	S = 52.8	mph
Space mean speed in outer lanes,	S = N/A	mph
Space mean speed for all vehicles,	S = 52.8	mph

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Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	2		
Free-flow speed on freeway	60.0	mph	
Volume on freeway	2240	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	40.0	mph	
Volume on ramp	910	vph	
Length of first accel/decel lane	250	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	No		
Volume on adjacent Ramp		vph	
Position of adjacent Ramp			
Type of adjacent Ramp			
Distance to adjacent Ramp		ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2240	910		vph
Peak-hour factor, PHF	0.91	0.94		
Peak 15-min volume, v15	615	242		v
Trucks and buses	5	1		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.976	0.995	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2523	973	pcph

Estimation of V12 Merge Areas

$$L = \frac{EQ}{P} \quad (\text{Equation 25-2 or 25-3})$$

$$P = 1.000 \quad \text{Using Equation 0}$$

$$v_{12} = v_{F \text{ FM}} (P) = 2523 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v	3496	4600	No
FO			
v	3496	4600	No
R12			

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 30.7 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence D

Speed Estimation

Intermediate speed variable,	M = 0.430	
Space mean speed in ramp influence area,	S = 52.3	mph
Space mean speed in outer lanes,	S = N/A	mph
Space mean speed for all vehicles,	S = 52.3	mph

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Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	60.0	mph
Volume on freeway	3150	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	45.0	mph
Volume on ramp	950	vph
Length of first accel/decel lane	325	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3150	950		vph
Peak-hour factor, PHF	0.91	0.91		
Peak 15-min volume, v15	865	261		v
Trucks and buses	5	0		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.976	1.000	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3548	1044	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)
EQ
P = 1.000 Using Equation 0
FD
 $v_{12} = v_R + (v_F - v_R) P_{FD} = 3548 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	3548	4600	No
v_{12}	3548	4400	No
$v_{FO} = v_F - v_R$	2504	4600	No
v_R	1044	2100	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 31.8 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence D

Speed Estimation

Intermediate speed variable,	D = 0.392	
Space mean speed in ramp influence area,	S = 52.9	mph
Space mean speed in outer lanes,	S = N/A	mph
Space mean speed for all vehicles,	S = 52.9	mph

HCS2000: Freeway Weaving Release 4.1f

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Operational Analysis

Analyst: NBS (Ex6_SB_AM)
Agency/Co.: VHB
Date Performed: 7/3/2012
Analysis Time Period: AM Peak Hour
Freeway/Dir of Travel: I-293 / SB
Weaving Location: Exit 6
Jurisdiction: NHDOT
Analysis Year: 2012 Existing Condition
Description: FEET I-293

Inputs

Freeway free-flow speed, SFF	60	mph
Weaving number of lanes, N	3	
Weaving segment length, L	500	ft
Terrain type	Level	
Grade		%
Length		mi
Weaving type	A	
Volume ratio, VR	0.43	
Weaving ratio, R	0.49	

Conversion to pc/h Under Base Conditions

	Non-Weaving		Weaving		
	V	V	V	V	
	A-C	B-D	A-D	B-C	
Volume, V	1835	5	650	715	veh/h
Peak-hour factor, PHF	0.89	0.93	0.80	0.93	
Peak 15-min volume, v15	515	2	203	192	v
Trucks and buses	4	1	0	1	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.980	0.995	1.000	0.995	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	2103	5	812	772	pc/h

Weaving and Non-Weaving Speeds

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	2.27	1.44
Weaving and non-weaving speeds, Si	30.30	35.52
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7)	1.32
Maximum number of lanes, Nw (max) (Exhibit 24-7)	1.40
Type of operation is	Unconstrained

 Weaving Segment Speed, Density, Level of Service and Capacity

Weaving segment speed, S	33.08	mph
Weaving segment density, D	37.20	pc/mi/ln
Level of service, LOS	E	
Capacity of base condition, cb	4099	pc/h
Capacity as a 15-minute flow rate, c	4019	pc/h
Capacity as a full-hour volume, ch	3540	pc/h

 Limitations on Weaving Segments

	Analyzed	If Max Exceeded	See Note
Weaving flow rate, Vw	1584	2800	a
Average flow rate (pcphpl)	1230	2300	b
Volume ratio, VR	0.43	0.45	c
Weaving ratio, R	0.49	N/A	d
Weaving length (ft)	500	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

HCS2000: Freeway Weaving Release 4.1f

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Operational Analysis

Analyst: NBS (Ex6_SB_PM)
Agency/Co.: VHB
Date Performed: 7/3/2012
Analysis Time Period: PM Peak Hour
Freeway/Dir of Travel: I-293 / SB
Weaving Location: Exit 6
Jurisdiction: NHDOT
Analysis Year: 2012 Existing Condition
Description: FEET I-293

Inputs

Freeway free-flow speed, SFF 60 mph
Weaving number of lanes, N 3
Weaving segment length, L 500 ft
Terrain type Level
Grade %
Length mi
Weaving type A
Volume ratio, VR 0.38
Weaving ratio, R 0.43

Conversion to pc/h Under Base Conditions

	Non-Weaving		Weaving		
	V	V	V	V	
	A-C	B-D	A-D	B-C	
Volume, V	1570	5	400	500	veh/h
Peak-hour factor, PHF	0.92	0.80	0.87	0.80	
Peak 15-min volume, v15	427	2	115	156	v
Trucks and buses	5	0	1	0	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.976	1.000	0.995	1.000	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	1749	6	462	624	pc/h

Weaving and Non-Weaving Speeds

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	1.63	0.89
Weaving and non-weaving speeds, Si	33.98	41.40
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7)	1.17
Maximum number of lanes, Nw (max) (Exhibit 24-7)	1.40
Type of operation is	Unconstrained

Weaving Segment Speed, Density, Level of Service and Capacity

Weaving segment speed, S	38.21	mph
Weaving segment density, D	24.78	pc/mi/ln
Level of service, LOS	C	
Capacity of base condition, cb	4235	pc/h
Capacity as a 15-minute flow rate, c	4132	pc/h
Capacity as a full-hour volume, ch	3667	pc/h

Limitations on Weaving Segments

	Analyzed	If Max Exceeded	See Note
Weaving flow rate, Vw	1086	2800	a
Average flow rate (pcphpl)	947	2300	b
Volume ratio, VR	0.38	0.45	c
Weaving ratio, R	0.43	N/A	d
Weaving length (ft)	500	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

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Merge Analysis

Analyst: NBS (Ex7_SB-On_AM)
 Agency/Co.: VHB
 Date performed: 7/31/2012
 Analysis time period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / SB
 Junction: Exit 7 SB On Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 No Build
 Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	2	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2300	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	825	vph
Length of first accel/decel lane	600	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2300	825		vph
Peak-hour factor, PHF	0.90	0.90		
Peak 15-min volume, v15	639	229		v
Trucks and buses	4	2		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.980	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2607	926	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
EQ
P = 1.000 Using Equation 0
FM
 $v_{12} = v_{F} (P_{FM}) = 2607 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v	3533	4600	No
FO			
v	3533	4600	No
R12			

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 28.8 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence D

Speed Estimation

Intermediate speed variable,	M = 0.412	
	S	
Space mean speed in ramp influence area,	S = 52.6	mph
	R	
Space mean speed in outer lanes,	S = N/A	mph
	0	
Space mean speed for all vehicles,	S = 52.6	mph

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Merge Analysis

Analyst: NBS (Ex6_SB-On_AM)
 Agency/Co.: VHB
 Date performed: 7/31/2012
 Analysis time period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / SB
 Junction: Exit 6 SB On Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 No Build
 Description: FEET I-293

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	2		
Free-flow speed on freeway	60.0	mph	
Volume on freeway	3205	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	485	vph	
Length of first accel/decel lane	300	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	825	vph	
Position of adjacent Ramp	Upstream		
Type of adjacent Ramp	Off		
Distance to adjacent Ramp	1500	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3205	485	825	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	890	135	229	v
Trucks and buses	4	2	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.980	0.990	1.000	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3632	544	917	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
EQ
P = 1.000 Using Equation 0
FM
 $v_{12} = v_F (P_{FM}) = 3632 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v FO	4176	4600	No
v R12	4176	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 35.9 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable,	M = 0.554	
Space mean speed in ramp influence area,	S = 50.0	mph
Space mean speed in outer lanes,	S = N/A	mph
Space mean speed for all vehicles,	S = 50.0	mph

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Diverge Analysis

Analyst: NBS (Ex6_NB-Off_AM)
Agency/Co.: VHB
Date performed: 7/31/2012
Analysis time period: AM Peak Hour
Freeway/Dir of Travel: I-293 / NB
Junction: Exit 6 NB Off Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 No Build
Description: FEET I-293

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	2		
Free-flow speed on freeway	60.0	mph	
Volume on freeway	2295	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	45.0	mph	
Volume on ramp	1200	vph	
Length of first accel/decel lane	275	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	No		
Volume on adjacent ramp		vph	
Position of adjacent ramp			
Type of adjacent ramp			
Distance to adjacent ramp		ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2295	1200		vph
Peak-hour factor, PHF	0.90	0.90		
Peak 15-min volume, v15	637	333		v
Trucks and buses	5	1		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.976	0.995	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2614	1340	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 1.000 \quad \text{Using Equation 0}$$

$$v_{12} = v_R + (v_F - v_R) \frac{P}{FD} = 2614 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2614	4600	No
v_{12}	2614	4400	No
$v_{FO} = v_F - v_R$	1274	4600	No
v_R	1340	2100	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 24.3 \quad \text{pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable,	$D = 0.419$	
Space mean speed in ramp influence area,	$S_R = 52.5$	mph
Space mean speed in outer lanes,	$S_0 = \text{N/A}$	mph
Space mean speed for all vehicles,	$S = 52.5$	mph

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Merge Analysis

Analyst: NBS (Ex6_NB-On_AM)
 Agency/Co.: VHB
 Date performed: 7/31/2012
 Analysis time period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 6 NB On Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 No Build
 Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	2	
Free-flow speed on freeway	60.0	mph
Volume on freeway	1095	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	40.0	mph
Volume on ramp	765	vph
Length of first accel/decel lane	250	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1095	765		vph
Peak-hour factor, PHF	0.90	0.90		
Peak 15-min volume, v15	304	212		v
Trucks and buses	5	2		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	%	%		%
Length	mi	mi		mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.976	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	1247	858	pcph

Estimation of V12 Merge Areas

$$L = \text{EQ} \quad (\text{Equation 25-2 or 25-3})$$

$$P = 1.000 \quad \text{Using Equation 0}$$

$$v_{12} = v_{F \text{ FM}} (P) = 1247 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	2105	4600	No
v _{R12}	2105	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 19.9 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	M = 0.333	
Space mean speed in ramp influence area,	S = 54.0	mph
Space mean speed in outer lanes,	S = N/A	mph
Space mean speed for all vehicles,	S = 54.0	mph

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Diverge Analysis

Analyst: NBS (Ex7_NB-Off_AM)
 Agency/Co.: VHB
 Date performed: 7/31/2012
 Analysis time period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 7 NB Off Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 No Build
 Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	60.0	mph
Volume on freeway	1860	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	45.0	mph
Volume on ramp	685	vph
Length of first accel/decel lane	325	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1860	685		vph
Peak-hour factor, PHF	0.90	0.90		
Peak 15-min volume, v15	517	190		v
Trucks and buses	5	2		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.976	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2118	769	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 1.000 \quad \text{Using Equation 0}$$

$$v_{12} = v_R + (v_F - v_R) \frac{P}{FD} = 2118 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{12} = v_{12}$	2118	4600	No
$v_{12} = v_{12}$	2118	4400	No
$v_{FO} = v_F - v_R$	1349	4600	No
v_R	769	2100	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 19.5 \quad \text{pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	$D = 0.367$	
Space mean speed in ramp influence area,	$S_R = 53.4$	mph
Space mean speed in outer lanes,	$S_0 = \text{N/A}$	mph
Space mean speed for all vehicles,	$S = 53.4$	mph

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Merge Analysis

Analyst: NBS (Ex7_SB-On_PM)
 Agency/Co.: VHB
 Date performed: 7/31/2012
 Analysis time period: PM Peak Hour
 Freeway/Dir of Travel: I-293 / SB
 Junction: Exit 7 SB On Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 No Build
 Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	2	
Free-flow speed on freeway	60.0	mph
Volume on freeway	1995	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	490	vph
Length of first accel/decel lane	600	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1995	490		vph
Peak-hour factor, PHF	0.90	0.90		
Peak 15-min volume, v15	554	136		v
Trucks and buses	5	2		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.976	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2272	550	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
EQ
P = 1.000 Using Equation 0
FM
 $v_{12} = v_{F} (P_{FM}) = 2272 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v	2822	4600	No
FO			
v	2822	4600	No
R12			

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 23.5 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable,	M = 0.345	
	S	
Space mean speed in ramp influence area,	S = 53.8	mph
	R	
Space mean speed in outer lanes,	S = N/A	mph
	O	
Space mean speed for all vehicles,	S = 53.8	mph

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Merge Analysis

Analyst: NBS (Ex6_SB-On_PM)
 Agency/Co.: VHB
 Date performed: 7/31/2012
 Analysis time period: PM Peak Hour
 Freeway/Dir of Travel: I-293 / SB
 Junction: Exit 6 SB On Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 No Build
 Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	2	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2610	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	420	vph
Length of first accel/decel lane	300	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	510	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1500	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2610	420	510	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	725	117	142	v
Trucks and buses	5	1	1	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.995	0.995	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2972	469	570	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
EQ
P = 1.000 Using Equation 0
FM
 $v_{12} = v_{F} (P_{FM}) = 2972 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v FO	3441	4600	No
v R12	3441	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 30.2 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence D

Speed Estimation

Intermediate speed variable,	M = 0.422	
Space mean speed in ramp influence area,	S = 52.4	mph
Space mean speed in outer lanes,	S = N/A	mph
Space mean speed for all vehicles,	S = 52.4	mph

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Diverge Analysis

Analyst: NBS (Ex6_NB-Off_PM)
 Agency/Co.: VHB
 Date performed: 7/31/2012
 Analysis time period: PM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 6 NB Off Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 No Build
 Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	60.0	mph
Volume on freeway	4160	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	45.0	mph
Volume on ramp	1345	vph
Length of first accel/decel lane	275	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4160	1345		vph
Peak-hour factor, PHF	0.90	0.90		
Peak 15-min volume, v15	1156	374		v
Trucks and buses	5	1		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.976	0.995	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4738	1502	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)
EQ
P = 1.000 Using Equation 0
FD
 $v_{12} = v_R + (v_F - v_R) P_{FD} = 4738 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	4738	4600	Yes
v_{12}	4738	4400	Yes
$v_{FO} = v_F - v_R$	3236	4600	No
v_R	1502	2100	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 42.5 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable,	D = 0.433	
Space mean speed in ramp influence area,	S = 52.2	mph
Space mean speed in outer lanes,	S = N/A	mph
Space mean speed for all vehicles,	S = 52.2	mph

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Merge Analysis

Analyst: NBS (Ex6_NB-On_PM)
Agency/Co.: VHB
Date performed: 7/31/2012
Analysis time period: PM Peak Hour
Freeway/Dir of Travel: I-293 / NB
Junction: Exit 6 NB On Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 No Build
Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	2	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2815	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	40.0	mph
Volume on ramp	1145	vph
Length of first accel/decel lane	250	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2815	1145		vph
Peak-hour factor, PHF	0.90	0.90		
Peak 15-min volume, v15	782	318		v
Trucks and buses	5	1		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.976	0.995	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3206	1279	pcph

Estimation of V12 Merge Areas

$$L = \text{EQ} \quad (\text{Equation 25-2 or 25-3})$$

$$P = 1.000 \quad \text{Using Equation 0}$$

$$v_{12} = v_{F \text{ FM}} (P) = 3206 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	4485	4600	No
v _{R12}	4485	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 38.3 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence E

Speed Estimation

Intermediate speed variable,	M = 0.647	
Space mean speed in ramp influence area,	S _R = 48.4	mph
Space mean speed in outer lanes,	S _O = N/A	mph
Space mean speed for all vehicles,	S = 48.4	mph

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Diverge Analysis

Analyst: NBS (Ex7_NB-Off_PM)
 Agency/Co.: VHB
 Date performed: 7/31/2012
 Analysis time period: PM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 7 NB Off Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 No Build
 Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	60.0	mph
Volume on freeway	3960	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	45.0	mph
Volume on ramp	1195	vph
Length of first accel/decel lane	325	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3960	1195		vph
Peak-hour factor, PHF	0.90	0.90		
Peak 15-min volume, v15	1100	332		v
Trucks and buses	5	0		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.976	1.000	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4510	1328	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 1.000 \quad \text{Using Equation 0}$$

$$v_{12} = v_R + (v_F - v_R) P = 4510 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{12} = v_{Fi}$	4510	4600	No
v_{12}	4510	4400	Yes
$v_{FO} = v_F - v_R$	3182	4600	No
v_R	1328	2100	No

Level of Service Determination (if not F)

$$\text{Density, } D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 40.1 \quad \text{pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence F

Speed Estimation

Intermediate speed variable,	$D = 0.418$	
Space mean speed in ramp influence area,	$S_R = 52.5$	mph
Space mean speed in outer lanes,	$S_0 = \text{N/A}$	mph
Space mean speed for all vehicles,	$S = 52.5$	mph

HCS2000: Freeway Weaving Release 4.1f

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Operational Analysis

Analyst: NBS (Ex6_SB_AM)
 Agency/Co.: VHB
 Date Performed: 7/31/2012
 Analysis Time Period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / SB
 Weaving Location: Exit 6
 Jurisdiction: NHDOT
 Analysis Year: 2035 No Build
 Description: FEET I-293

Inputs

Freeway free-flow speed, SFF	60	mph
Weaving number of lanes, N	3	
Weaving segment length, L	500	ft
Terrain type	Level	
Grade		%
Length		mi
Weaving type	A	
Volume ratio, VR	0.42	
Weaving ratio, R	0.48	

Conversion to pc/h Under Base Conditions

	Non-Weaving		Weaving		
	V	V	V	V	
	A-C	B-D	A-D	B-C	
Volume, V	2305	5	820	900	veh/h
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	
Peak 15-min volume, v15	640	2	228	250	v
Trucks and buses	4	1	0	1	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.980	0.995	1.000	0.995	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	2612	5	911	1004	pc/h

Weaving and Non-Weaving Speeds

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	2.74	1.84
Weaving and non-weaving speeds, Si	28.38	32.60
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7)	1.34
Maximum number of lanes, Nw (max) (Exhibit 24-7)	1.40
Type of operation is	Unconstrained

Weaving Segment Speed, Density, Level of Service and Capacity

Weaving segment speed, S	30.67	mph
Weaving segment density, D	49.25	pc/mi/ln
Level of service, LOS	F	
Capacity of base condition, cb	4117	pc/h
Capacity as a 15-minute flow rate, c	4036	pc/h
Capacity as a full-hour volume, ch	3632	pc/h

Limitations on Weaving Segments

	Analyzed	If Max Exceeded	See Note
Weaving flow rate, Vw	1915	2800	a
Average flow rate (pcphpl)	1510	2300	b
Volume ratio, VR	0.42	0.45	c
Weaving ratio, R	0.48	N/A	d
Weaving length (ft)	500	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

HCS2000: Freeway Weaving Release 4.1f

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Operational Analysis

Analyst: NBS (Ex6_SB_PM)
 Agency/Co.: VHB
 Date Performed: 7/31/2012
 Analysis Time Period: PM Peak Hour
 Freeway/Dir of Travel: I-293 / SB
 Weaving Location: Exit 6
 Jurisdiction: NHDOT
 Analysis Year: 2035 No Build
 Description: FEET I-293

Inputs

Freeway free-flow speed, SFF	60	mph
Weaving number of lanes, N	3	
Weaving segment length, L	500	ft
Terrain type	Level	
Grade		%
Length		mi
Weaving type	A	
Volume ratio, VR	0.36	
Weaving ratio, R	0.45	

Conversion to pc/h Under Base Conditions

	Non-Weaving		Weaving		
	V	V	V	V	
	A-C	B-D	A-D	B-C	
Volume, V	1980	5	505	630	veh/h
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	
Peak 15-min volume, v15	550	2	140	175	v
Trucks and buses	5	0	1	0	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.976	1.000	0.995	1.000	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	2255	5	563	700	pc/h

Weaving and Non-Weaving Speeds

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	1.94	1.10
Weaving and non-weaving speeds, Si	32.02	38.77
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7)	1.16
Maximum number of lanes, Nw (max) (Exhibit 24-7)	1.40
Type of operation is	Unconstrained

Weaving Segment Speed, Density, Level of Service and Capacity

Weaving segment speed, S	36.04	mph
Weaving segment density, D	32.58	pc/mi/ln
Level of service, LOS	D	
Capacity of base condition, cb	4309	pc/h
Capacity as a 15-minute flow rate, c	4204	pc/h
Capacity as a full-hour volume, ch	3784	pc/h

Limitations on Weaving Segments

	Analyzed	If Max Exceeded	See Note
Weaving flow rate, Vw	1263	2800	a
Average flow rate (pcphpl)	1174	2300	b
Volume ratio, VR	0.36	0.45	c
Weaving ratio, R	0.45	N/A	d
Weaving length (ft)	500	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

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Diverge Analysis

Analyst: NBS (Alt-4&8_Ex6_NB-Off_AM)
 Agency/Co.: VHB
 Date performed: 3/19/2013
 Analysis time period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 6 NB Off Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt-4&8
 Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2270	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1200	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	835	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	3100	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2270	1200	835	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	631	333	232	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2585	1347	937	pcph

Estimation of V12 Diverge Areas

$$L = \text{EQ} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.633 \quad \text{Using Equation 5}$$

$$v_{12} = v_R + (v_F - v_R) P = 2131 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2585	6900	No
v_{12}	2131	4400	No
$v_{FO} = v_F - v_R$	1238	6900	No
v_R	1347	2000	No

Level of Service Determination (if not F)

$$\text{Density, } D = 4.252 + 0.0086 v_{12} - 0.009 L = 18.2 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	$D = 0.549$	
Space mean speed in ramp influence area,	$S = 50.1$	mph
Space mean speed in outer lanes,	$S = 65.8$	mph
Space mean speed for all vehicles,	$S = 52.3$	mph

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Diverge Analysis

Analyst: NBS (Alt-4&9_Ex6_NB-Off_AM)
 Agency/Co.: VHB
 Date performed: 3/19/2013
 Analysis time period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 6 NB Off Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt-4&9
 Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2270	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1200	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	685	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	3100	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2270	1200	685	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	631	333	190	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2585	1347	769	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.633 \quad \text{Using Equation 5}$$

$$v_{12} = v_R + (v_F - v_R) P = 2131 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2585	6900	No
v_{12}	2131	4400	No
$v_{FO} = v_F - v_R$	1238	6900	No
v_R	1347	2000	No

Level of Service Determination (if not F)

$$\text{Density, } D = 4.252 + 0.0086 v_{12} - 0.009 L = 18.2 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	D = 0.549	
Space mean speed in ramp influence area,	S = 50.1	mph
Space mean speed in outer lanes,	S = 65.8	mph
Space mean speed for all vehicles,	S = 52.3	mph

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Diverge Analysis

Analyst: NBS (Alt-4&10_Ex6_NB-Off_AM)
 Agency/Co.: VHB
 Date performed: 3/19/2013
 Analysis time period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 6 NB Off Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt-4&10
 Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2270	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1180	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	575	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	3100	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2270	1180	575	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	631	328	160	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2585	1324	645	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad \text{(Equation 25-8 or 25-9)}$$

$$P = 0.634 \quad \text{Using Equation 5}$$

$$v_{12} = v_R + (v_F - v_R) P = 2124 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2585	6900	No
v_{12}	2124	4400	No
$v_{FO} = v_F - v_R$	1261	6900	No
v_R	1324	2000	No

Level of Service Determination (if not F)

$$\text{Density, } D = 4.252 + 0.0086 v_{12} - 0.009 L = 18.1 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	D = 0.547	
Space mean speed in ramp influence area,	S = 50.2	mph
Space mean speed in outer lanes,	S = 65.8	mph
Space mean speed for all vehicles,	S = 52.4	mph

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Diverge Analysis

Analyst: NBS (Alt-4&8_Ex6_NB-Off_PM)
 Agency/Co.: VHB
 Date performed: 3/19/2013
 Analysis time period: PM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 6 NB Off Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt-4&8
 Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	4000	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1350	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	1350	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	3100	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4000	1350	1350	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	1111	375	375	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	4556	1515	1515	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.576 \quad \text{Using Equation 5}$$

$$FD$$

$$v_{12} = v_R + (v_F - v_R) P = 3268 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	4556	6900	No
v_{12}	3268	4400	No
$v_{FO} = v_F - v_R$	3041	6900	No
v_R	1515	2000	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 28.0 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable,	$D = 0.564$	
Space mean speed in ramp influence area,	$S_R = 49.8$	mph
Space mean speed in outer lanes,	$S_O = 64.7$	mph
Space mean speed for all vehicles,	$S = 53.3$	mph

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Diverge Analysis

Analyst: NBS (Alt-4&9_Ex6_NB-Off_PM)
Agency/Co.: VHB
Date performed: 3/19/2013
Analysis time period: PM Peak Hour
Freeway/Dir of Travel: I-293 / NB
Junction: Exit 6 NB Off Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt-4&9
Description: FEET I-293

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	3		
Free-flow speed on freeway	60.0	mph	
Volume on freeway	4000	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	1350	vph	
Length of first accel/decel lane	486	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent ramp	1150	vph	
Position of adjacent ramp	Downstream		
Type of adjacent ramp	On		
Distance to adjacent ramp	3100	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4000	1350	1150	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	1111	375	319	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	4556	1515	1291	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} = \frac{0.576}{0.576} \text{ Using Equation 5}$$

$$v_{12} = v_R + (v_F - v_R) P_{FD} = 3268 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	4556	6900	No
v_{12}	3268	4400	No
$v_{FO} = v_F - v_R$	3041	6900	No
v_R	1515	2000	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 28.0 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable,	$D = 0.564$	
Space mean speed in ramp influence area,	$S = 49.8$	mph
Space mean speed in outer lanes,	$S = 64.7$	mph
Space mean speed for all vehicles,	$S = 53.3$	mph

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Diverge Analysis

Analyst: NBS (Alt-4&10_Ex6_NB-Off_PM)
Agency/Co.: VHB
Date performed: 3/19/2013
Analysis time period: PM Peak Hour
Freeway/Dir of Travel: I-293 / NB
Junction: Exit 6 NB Off Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt-4&10
Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	4000	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1295	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	1035	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	3100	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4000	1295	1035	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	1111	360	288	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	4556	1453	1162	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.579 \quad \text{Using Equation 5}$$

$$FD$$

$$v_{12} = v_R + (v_F - v_R) P = 3250 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	4556	6900	No
v_{12}	3250	4400	No
$v_{FO} = v_F - v_R$	3103	6900	No
v_R	1453	2000	No

Level of Service Determination (if not F)

$$\text{Density, } D = 4.252 + 0.0086 v_{12} - 0.009 L = 27.8 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable,	$D = 0.559$	
Space mean speed in ramp influence area,	$S = 49.9$	mph
Space mean speed in outer lanes,	$S = 64.6$	mph
Space mean speed for all vehicles,	$S = 53.4$	mph

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Merge Analysis

Analyst: NBS (Alt-4&9_Ex6_NB-On_AM)
 Agency/Co.: VHB
 Date performed: 3/19/2013
 Analysis time period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 6 NB On Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt-4&9
 Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	1070	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	685	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1200	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	3100	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1070	685	1200	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	297	190	333	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1219	769	1347	pcph

Estimation of V12 Merge Areas

$$L_{EQ} = 208.83 \quad (\text{Equation 25-2 or 25-3})$$

$$P_{FM} = 0.600 \quad \text{Using Equation 1}$$

$$v_{12} = v_{F, FM} (P_{FM}) = 731 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	1988	6900	No
v _{R12}	1500	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 11.8 \quad \text{pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	M = 0.282	
Space mean speed in ramp influence area,	S _R = 54.9	mph
Space mean speed in outer lanes,	S ₀ = 60.0	mph
Space mean speed for all vehicles,	S = 56.1	mph

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Merge Analysis

Analyst: NBS (Alt-4&10_Ex6_NB-On_AM)
 Agency/Co.: VHB
 Date performed: 3/19/2013
 Analysis time period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 6 NB On Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt-4&10
 Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	1090	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	575	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1180	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	3100	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1090	575	1180	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	303	160	328	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1241	645	1324	pcph

Estimation of V12 Merge Areas

L = 187.00 (Equation 25-2 or 25-3)
EQ
P = 0.600 Using Equation 1
FM
 $v_{12} = v_{F} (P_{FM}) = 744$ pc/h

Capacity Checks

	Actual	Maximum	LOS F?
v FO	1886	6900	No
v R12	1389	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 11.0$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, M = 0.281
S
Space mean speed in ramp influence area, S = 54.9 mph
R
Space mean speed in outer lanes, S = 60.0 mph
O
Space mean speed for all vehicles, S = 56.2 mph

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Merge Analysis

Analyst: NBS (Alt-4&9_Ex6_NB-On_PM)
 Agency/Co.: VHB
 Date performed: 3/19/2013
 Analysis time period: PM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 6 NB On Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt-4&9
 Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2650	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	1150	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1350	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	3100	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2650	1150	1350	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	736	319	375	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3018	1291	1515	pcph

Estimation of V12 Merge Areas

$$L = 705.53 \quad (\text{Equation 25-2 or 25-3})$$

EQ

$$P = 0.600 \quad \text{Using Equation 1}$$

FM

$$v_{12} = v_{F \text{ FM}} (P) = 1810 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	4309	6900	No
v _{R12}	3101	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 24.1 \quad \text{pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable,	M = 0.352	
	S	
Space mean speed in ramp influence area,	S = 53.7	mph
	R	
Space mean speed in outer lanes,	S = 57.5	mph
	O	
Space mean speed for all vehicles,	S = 54.7	mph

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Merge Analysis

Analyst: NBS (Alt-4&10_Ex6_NB-On_PM)
 Agency/Co.: VHB
 Date performed: 3/19/2013
 Analysis time period: PM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 6 NB On Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt-4&10
 Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2705	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	1035	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1295	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	3100	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2705	1035	1295	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	751	287	360	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3081	1161	1453	pcph

Estimation of V12 Merge Areas

$$L = 691.19 \quad (\text{Equation 25-2 or 25-3})$$

EQ

$$P = 0.600 \quad \text{Using Equation 1}$$

FM

$$v_{12} = v_{F \text{ FM}} = 1848 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v FO	4242	6900	No
v R12	3009	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 23.4 \quad \text{pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable,	M = 0.344	
	S	
Space mean speed in ramp influence area,	S = 53.8	mph
	R	
Space mean speed in outer lanes,	S = 57.4	mph
	O	
Space mean speed for all vehicles,	S = 54.8	mph

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Diverge Analysis

Analyst: NBS (Alt-4&9_Ex6_SB-Off_AM)
Agency/Co.: VHB
Date performed: 3/19/2013
Analysis time period: AM Peak Hour
Freeway/Dir of Travel: I-293 / SB
Junction: Exit 6 SB Off Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt-4&9
Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	3005	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1100	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	485	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	4000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3005	1100	485	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	835	306	135	v
Trucks and buses	4	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.980	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3406	1234	544	pcph

Estimation of V12 Diverge Areas

$$L = \text{EQ} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.618 \quad \text{Using Equation 5}$$

$$v_{12} = v_R + (v_F - v_R) P = 2576 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	3406	6900	No
v_{12}	2576	4400	No
$v_{FO} = v_F - v_R$	2172	6900	No
v_R	1234	2000	No

Level of Service Determination (if not F)

$$\text{Density, } D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 22.0 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable,	$D = 0.539$	
Space mean speed in ramp influence area,	$S_R = 50.3$	mph
Space mean speed in outer lanes,	$S_O = 65.8$	mph
Space mean speed for all vehicles,	$S = 53.4$	mph

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Diverge Analysis

Analyst: NBS (Alt-4&10_Ex6_SB-Off_AM)
Agency/Co.: VHB
Date performed: 3/19/2013
Analysis time period: AM Peak Hour
Freeway/Dir of Travel: I-293 / SB
Junction: Exit 6 SB Off Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt-4&10
Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2955	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1050	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	485	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	4000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2955	1050	485	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	821	292	135	v
Trucks and buses	4	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.980	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3349	1178	544	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.622 \quad \text{Using Equation 5}$$

$$v_{12} = v_R + (v_F - v_R) P = 2529 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	3349	6900	No
v_{12}	2529	4400	No
$v_{FO} = v_F - v_R$	2171	6900	No
v_R	1178	2000	No

Level of Service Determination (if not F)

$$\text{Density, } D = 4.252 + 0.0086 v_{12} - 0.009 L = 21.6 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable,	D = 0.534	
Space mean speed in ramp influence area,	S = 50.4	mph
Space mean speed in outer lanes,	S = 65.8	mph
Space mean speed for all vehicles,	S = 53.5	mph

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Diverge Analysis

Analyst: NBS (Alt-4&9_Ex6_SB-Off_PM)
Agency/Co.: VHB
Date performed: 3/19/2013
Analysis time period: PM Peak Hour
Freeway/Dir of Travel: I-293 / SB
Junction: Exit 6 SB Off Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt-4&9
Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2385	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	530	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	420	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	4000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2385	530	420	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	662	147	117	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2716	595	471	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.665 \quad \text{Using Equation 5}$$

$$FD$$

$$v_{12R} = v_F + (v_F - v_R) P = 2005 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2716	6900	No
v_{12}	2005	4400	No
$v_{FO} = v_F - v_R$	2121	6900	No
v_R	595	2000	No

Level of Service Determination (if not F)

$$D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 17.1 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	$D = 0.482$	
	S	
Space mean speed in ramp influence area,	$S = 51.3$	mph
	R	
Space mean speed in outer lanes,	$S = 65.8$	mph
	O	
Space mean speed for all vehicles,	$S = 54.5$	mph

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Diverge Analysis

Analyst: NBS (Alt-4&10_Ex6_SB-Off_PM)
Agency/Co.: VHB
Date performed: 3/19/2013
Analysis time period: PM Peak Hour
Freeway/Dir of Travel: I-293 / SB
Junction: Exit 6 SB Off Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt-4&10
Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2300	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	470	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	420	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	4000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2300	470	420	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	639	131	117	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2619	527	471	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad \text{(Equation 25-8 or 25-9)}$$

$$P = 0.670 \quad \text{Using Equation 5}$$

$$v_{12} = v_R + (v_F - v_R) P = 1929 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2619	6900	No
v_{12}	1929	4400	No
$v_{FO} = v_F - v_R$	2092	6900	No
v_R	527	2000	No

Level of Service Determination (if not F)

$$\text{Density, } D = 4.252 + 0.0086 v_{12} - 0.009 L = 16.5 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	$D = 0.475$	
Space mean speed in ramp influence area,	$S_R = 51.4$	mph
Space mean speed in outer lanes,	$S_0 = 65.8$	mph
Space mean speed for all vehicles,	$S = 54.6$	mph

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Merge Analysis

Analyst: NBS (Alt-4&8_Ex6_SB-On)
Agency/Co.: VBB
Date performed: 3/19/2013
Analysis time period: AM Peak
Freeway/Dir of Travel: I-293 / Southbound
Junction: Exit 6 SB On Ramp (Eddy Rd NB)
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt-4&8
Description: I-293 #52196

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	60.0	mph	
Volume on freeway	3230	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	485	vph	
Length of first accel/decel lane	800	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	1325	vph	
Position of adjacent Ramp	Upstream		
Type of adjacent Ramp	On		
Distance to adjacent Ramp	1100	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3230	485	1325	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	897	135	368	v
Trucks and buses	4	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.980	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3661	544	1487	pcph

Estimation of V12 Merge Areas

$$L = \text{(Equation 25-2 or 25-3)}$$

$$EQ$$

$$P = 0.600 \quad \text{Using Equation 1}$$

$$FM$$

$$v_{12} = v_F (P_{FM}) = 2196 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	4205	6900	No
v _{R12}	2740	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 21.6 \quad \text{pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable,	M = 0.325	
	S	
Space mean speed in ramp influence area,	S = 54.1	mph
	R	
Space mean speed in outer lanes,	S = 56.5	mph
	O	
Space mean speed for all vehicles,	S = 54.9	mph

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Merge Analysis

Analyst: NBS (Alt-4&9_Ex6_SB-On)
 Agency/Co.: VBB
 Date performed: 3/19/2013
 Analysis time period: AM Peak
 Freeway/Dir of Travel: I-293 / Southbound
 Junction: Exit 6 SB On Ramp (Eddy Rd NB)
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt-4&9
 Description: I-293 #52196

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	3230	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	485	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1325	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1100	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3230	485	1325	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	897	135	368	v
Trucks and buses	4	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.980	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3661	544	1487	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
EQ
P = 0.600 Using Equation 1
FM
 $v_{12} = v_{F \text{ FM}} = 2196 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v FO	4205	6900	No
v R12	2740	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 21.6 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable,	M = 0.325	
	S	
Space mean speed in ramp influence area,	S = 54.1	mph
	R	
Space mean speed in outer lanes,	S = 56.5	mph
	O	
Space mean speed for all vehicles,	S = 54.9	mph

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Merge Analysis

Analyst: NBS (Alt-4&10_Ex6_SB-On)
 Agency/Co.: VBB
 Date performed: 3/19/2013
 Analysis time period: AM Peak
 Freeway/Dir of Travel: I-293 / Southbound
 Junction: Exit 6 SB On Ramp (Eddy Rd NB)
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt-4&10
 Description: I-293 #52196

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	3180	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	485	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1275	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1100	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3180	485	1275	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	883	135	354	v
Trucks and buses	4	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.980	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3604	544	1431	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
EQ
P = 0.600 Using Equation 1
FM
 $v_{12} = v_F (P_{FM}) = 2162 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v FO	4148	6900	No
v R12	2706	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 21.3 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable,	M = 0.323	
Space mean speed in ramp influence area,	S = 54.2	mph
Space mean speed in outer lanes,	S = 56.6	mph
Space mean speed for all vehicles,	S = 55.0	mph

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Merge Analysis

Analyst: NBS (Alt-4&8_Ex6_SB-On_PM)
 Agency/Co.: VBB
 Date performed: 3/19/2013
 Analysis time period: PM Peak
 Freeway/Dir of Travel: I-293 / Southbound
 Junction: Exit 6 SB On Ramp (Eddy Rd NB)
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt-4&8
 Description: I-293 #52196

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2490	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	420	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	635	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1100	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2490	420	635	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	692	117	176	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2836	471	713	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
EQ
P = 0.600 Using Equation 1
FM
 $v_{12} = v_{F} (P_{FM}) = 1701 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	3307	6900	No
v _{R12}	2172	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 17.2 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	M = 0.299	
	S	
Space mean speed in ramp influence area,	S = 54.6	mph
	R	
Space mean speed in outer lanes,	S = 57.7	mph
	O	
Space mean speed for all vehicles,	S = 55.6	mph

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Merge Analysis

Analyst: NBS (Alt-4&9_Ex6_SB-On_PM)
 Agency/Co.: VBB
 Date performed: 3/19/2013
 Analysis time period: PM Peak
 Freeway/Dir of Travel: I-293 / Southbound
 Junction: Exit 6 SB On Ramp (Eddy Rd NB)
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt-4&9
 Description: I-293 #52196

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2490	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	420	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	635	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1100	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2490	420	635	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	692	117	176	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2836	471	713	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
EQ
P = 0.600 Using Equation 1
FM
 $v_{12} = v_F (P_{FM}) = 1701 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	3307	6900	No
v _{R12}	2172	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 17.2 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	M = 0.299	
Space mean speed in ramp influence area,	S _R = 54.6	mph
Space mean speed in outer lanes,	S ₀ = 57.7	mph
Space mean speed for all vehicles,	S = 55.6	mph

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Merge Analysis

Analyst: NBS (Alt-4&10_Ex6_SB-On_PM)
 Agency/Co.: VBB
 Date performed: 3/19/2013
 Analysis time period: PM Peak
 Freeway/Dir of Travel: I-293 / Southbound
 Junction: Exit 6 SB On Ramp (Eddy Rd NB)
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt-4&10
 Description: I-293 #52196

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2440	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	420	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	610	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1100	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2440	420	610	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	678	117	169	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2779	471	685	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
EQ
P = 0.600 Using Equation 1
FM
 $v_{12} = v_F (P_{FM}) = 1667 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v FO	3250	6900	No
v R12	2138	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 16.9 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	M = 0.298	
Space mean speed in ramp influence area,	S = 54.6	mph
Space mean speed in outer lanes,	S = 57.8	mph
Space mean speed for all vehicles,	S = 55.7	mph

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Diverge Analysis

Analyst: NBS (Alt-567&9_Ex6_NB-Off_AM)
Agency/Co.: VHB
Date performed: 3/19/2013
Analysis time period: AM Peak Hour
Freeway/Dir of Travel: I-293 / NB
Junction: Exit 6 NB Off Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt-5,6,7&9
Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2270	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1200	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	320	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	1700	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2270	1200	320	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	631	333	89	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2585	1347	359	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.633 \quad \text{Using Equation 5}$$

$$FD$$

$$v_{12R} = v_F + (v_R - v_F) P = 2131 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2585	6900	No
v_{12}	2131	4400	No
$v_{FO} = v_F - v_R$	1238	6900	No
v_R	1347	2000	No

Level of Service Determination (if not F)

$$\text{Density, } D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 18.2 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	D = 0.549	
	S	
Space mean speed in ramp influence area,	S = 50.1	mph
	R	
Space mean speed in outer lanes,	S = 65.8	mph
	O	
Space mean speed for all vehicles,	S = 52.3	mph

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Diverge Analysis

Analyst: NBS (Alt-567&9_Ex6_NB-Off_PM)
Agency/Co.: VHB
Date performed: 3/19/2013
Analysis time period: PM Peak Hour
Freeway/Dir of Travel: I-293 / NB
Junction: Exit 6 NB Off Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt-5,6,7&9
Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	4000	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1350	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	535	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	1700	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4000	1350	535	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	1111	375	149	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	4556	1515	600	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.576 \quad \text{Using Equation 5}$$

$$v_{12} = v_R + (v_F - v_R) P = 3268 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	4556	6900	No
v_{12}	3268	4400	No
$v_{FO} = v_F - v_R$	3041	6900	No
v_R	1515	2000	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 28.0 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable,	$D = 0.564$	
Space mean speed in ramp influence area,	$S_R = 49.8$	mph
Space mean speed in outer lanes,	$S_O = 64.7$	mph
Space mean speed for all vehicles,	$S = 53.3$	mph

HCS2000: Ramps and Ramp Junctions Release 4.1f

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Merge Analysis

Analyst: NBS (Alt-567&9_Ex6_NB-On1_AM)
Agency/Co.: VHB
Date performed: 3/19/2013
Analysis time period: AM Peak Hour
Freeway/Dir of Travel: I-293 / NB
Junction: Exit 6 NB On Ramp (first ramp)
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt-5,6,7&9
Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	1070	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	320	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1200	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1700	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1070	320	1200	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	297	89	333	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1219	359	1347	pcph

Estimation of V12 Merge Areas

L = 121.09 (Equation 25-2 or 25-3)
EQ
P = 0.600 Using Equation 1
FM
 $v_{12} = v_{F \text{ FM}} = 731 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v FO	1578	6900	No
v R12	1090	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 8.8 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence A

Speed Estimation

Intermediate speed variable, M = 0.277
S
Space mean speed in ramp influence area, S = 55.0 mph
R
Space mean speed in outer lanes, S = 60.0 mph
O
Space mean speed for all vehicles, S = 56.5 mph

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Merge Analysis

Analyst: NBS (Alt-567&9_Ex6_NB-On1_PM)
 Agency/Co.: VHB
 Date performed: 3/19/2013
 Analysis time period: PM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 6 NB On Ramp (first ramp)
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt-5,6,7&9
 Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2650	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	535	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1350	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1700	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2650	535	1350	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	736	149	375	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3018	600	1515	pcph

Estimation of V12 Merge Areas

$$L = 557.65 \quad (\text{Equation 25-2 or 25-3})$$

EQ

$$P = 0.600 \quad \text{Using Equation 1}$$

FM

$$v_{12} = v_F(P_{FM}) = 1810 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	3618	6900	No
v _{R12}	2410	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 19.0 \quad \text{pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	M = 0.308	
	S	
Space mean speed in ramp influence area,	S = 54.4	mph
	R	
Space mean speed in outer lanes,	S = 57.5	mph
	O	
Space mean speed for all vehicles,	S = 55.4	mph

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Merge Analysis

Analyst: NBS (Alt-567&9_Ex6_NB-On2_AM)
 Agency/Co.: VHB
 Date performed: 3/19/2013
 Analysis time period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 6 NB On Ramp (2nd ramp)
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt-5,6,7&9
 Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	1390	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	365	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	320	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1700	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1390	365	320	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	386	101	89	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1583	410	359	pcph

Estimation of V12 Merge Areas

L = 209.90 (Equation 25-2 or 25-3)
EQ
P = 0.600 Using Equation 1
FM
 $v_{12} = v_{F \text{ FM}} = 950 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v FO	1993	6900	No
v R12	1360	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 10.9 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	M = 0.280	
	S	
Space mean speed in ramp influence area,	S = 55.0	mph
	R	
Space mean speed in outer lanes,	S = 59.5	mph
	O	
Space mean speed for all vehicles,	S = 56.3	mph

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Merge Analysis

Analyst: NBS (Alt-567&9_Ex6_NB-On2_PM)
 Agency/Co.: VHB
 Date performed: 3/19/2013
 Analysis time period: PM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 6 NB On Ramp (2nd ramp)
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt-5,6,7&9
 Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	3185	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	615	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	535	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1700	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3185	615	535	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	885	171	149	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3627	690	600	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
EQ
P = 0.600 Using Equation 1
FM
 $v_{12} = v_{F} (P_{FM}) = 2176 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v FO	4317	6900	No
v R12	2866	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 22.5 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable,	M = 0.334	
Space mean speed in ramp influence area,	S = 54.0	mph
Space mean speed in outer lanes,	S = 56.6	mph
Space mean speed for all vehicles,	S = 54.8	mph

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Diverge Analysis

Analyst: NBS (Alt-567&9_Ex6_SB-Off_AM)
 Agency/Co.: VHB
 Date performed: 3/19/2013
 Analysis time period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / SB
 Junction: Exit 6 SB Off Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt-5,6,7&9
 Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	3005	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1100	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	1810	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	2100	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3005	1100	1810	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	835	306	503	v
Trucks and buses	4	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.980	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3406	1234	2031	pcph

Estimation of V12 Diverge Areas

$$L = \text{EQ} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.618 \quad \text{Using Equation 5}$$

$$v_{12} = v_R + (v_F - v_R) P = 2576 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS, F?
$v_{Fi} = v_F$	3406	6900	No
v_{12}	2576	4400	No
$v_{FO} = v_F - v_R$	2172	6900	No
v_R	1234	2000	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 22.0 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable,	$D = 0.539$	
Space mean speed in ramp influence area,	$S_R = 50.3$	mph
Space mean speed in outer lanes,	$S_O = 65.8$	mph
Space mean speed for all vehicles,	$S = 53.4$	mph

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Diverge Analysis

Analyst: NBS (Alt-567&9_Ex6_SB-Off_PM)
 Agency/Co.: VHB
 Date performed: 3/19/2013
 Analysis time period: PM Peak Hour
 Freeway/Dir of Travel: I-293 / SB
 Junction: Exit 6 SB Off Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt-5,6,7&9
 Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2405	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	530	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	1055	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	2100	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2405	530	1055	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	668	147	293	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2739	595	1184	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.664 \quad \text{Using Equation 5}$$

$$FD$$

$$v_{12} = v_R + (v_F - v_R) P = 2019 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2739	6900	No
v_{12}	2019	4400	No
$v_{FO} = v_F - v_R$	2144	6900	No
v_R	595	2000	No

Level of Service Determination (if not F)

$$\text{Density, } D = 4.252 + 0.0086 v_{12} - 0.009 L = 17.2 \text{ pc/mi/ln}$$

$$\text{Level of service for ramp-freeway junction areas of influence B}$$

Speed Estimation

Intermediate speed variable,	$D = 0.482$	
Space mean speed in ramp influence area,	$S_R = 51.3$	mph
Space mean speed in outer lanes,	$S_0 = 65.8$	mph
Space mean speed for all vehicles,	$S = 54.5$	mph

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Diverge Analysis

Analyst: NBS (Alt-9&4567_Ex7_NB-Off_AM)
Agency/Co.: VHB
Date performed: 3/20/2013
Analysis time period: AM Peak Hour
Freeway/Dir of Travel: I-293 / NB
Junction: Exit 7 NB Off Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt-9 & 4,5,6,7
Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	1755	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	570	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	305	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	2400	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1755	570	305	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	487	158	85	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1999	640	342	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.681 \quad \text{Using Equation 5}$$

$$v_{12} = v_R + (v_F - v_R) P = 1565 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	1999	6900	No
v_{12}	1565	4400	No
$v_{FO} = v_F - v_R$	1359	6900	No
v_R	640	2000	No

Level of Service Determination (if not F)

$$\text{Density, } D = 4.252 + 0.0086 v_{12} - 0.009 L = 13.3 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	$D = 0.486$	
Space mean speed in ramp influence area,	$S_R = 51.3$	mph
Space mean speed in outer lanes,	$S_O = 65.8$	mph
Space mean speed for all vehicles,	$S = 53.8$	mph

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Diverge Analysis

Analyst: NBS (Alt-9&4567_Ex7_NB-Off_PM)
 Agency/Co.: VHB
 Date performed: 3/20/2013
 Analysis time period: PM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 7 NB Off Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt-9 & 4,5,6,7
 Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	3800	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1000	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	360	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	2400	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3800	1000	360	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	1056	278	100	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	4328	1122	404	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.600 \quad \text{Using Equation 5}$$

$$v_{12} = v_R + (v_F - v_R) P = 3046 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	4328	6900	No
v_{12}	3046	4400	No
$v_{FO} = v_F - v_R$	3206	6900	No
v_R	1122	2000	No

Level of Service Determination (if not F)

$$\text{Density, } D = 4.252 + 0.0086 v_{12} - 0.009 L = 26.1 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable,	D = 0.529	
Space mean speed in ramp influence area,	S _R = 50.5	mph
Space mean speed in outer lanes,	S _O = 64.7	mph
Space mean speed for all vehicles,	S ₀ = 54.0	mph

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Diverge Analysis

Analyst: NBS (Alt-10&4_Ex7_NB-Off_AM)
 Agency/Co.: VHB
 Date performed: 3/20/2013
 Analysis time period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 7 NB Off Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt 10 & 4
 Description: FEET I-293

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	3		
Free-flow speed on freeway	60.0	mph	
Volume on freeway	1665	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	590	vph	
Length of first accel/decel lane	486	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent ramp	565	vph	
Position of adjacent ramp	Downstream		
Type of adjacent ramp	On		
Distance to adjacent ramp	2400	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1665	590	565	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	462	164	157	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1896	662	634	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.682 \quad \text{Using Equation 5}$$

$$FD$$

$$v_{12R} = v_F + (v_F - v_R) P = 1504 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	1896	6900	No
v_{12}	1504	4400	No
$v_{FO} = v_F - v_R$	1234	6900	No
v_R	662	2000	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 12.8 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	$D = 0.488$	
Space mean speed in ramp influence area,	$S_R = 51.2$	mph
Space mean speed in outer lanes,	$S_0 = 65.8$	mph
Space mean speed for all vehicles,	$S = 53.7$	mph

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Diverge Analysis

Analyst: NBS (Alt-10&4_Ex7_NB-Off_PM)
Agency/Co.: VHB
Date performed: 3/20/2013
Analysis time period: PM Peak Hour
Freeway/Dir of Travel: I-293 / NB
Junction: Exit 7 NB Off Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt 10 & 4
Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	3740	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1055	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	575	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	2400	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3740	1055	575	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	1039	293	160	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	4259	1184	645	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.599 \quad \text{Using Equation 5}$$

$$FD$$

$$v_{12R} = v_F + (v_R - v_F) P = 3026 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	4259	6900	No
v_{12}	3026	4400	No
$v_{FO} = v_F - v_R$	3075	6900	No
v_R	1184	2000	No

Level of Service Determination (if not F)

$$\text{Density, } D = 4.252 + 0.0086 v_{12} - 0.009 L = 25.9 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable,	$D = 0.535$	
	S	
Space mean speed in ramp influence area,	$S = 50.4$	mph
	R	
Space mean speed in outer lanes,	$S = 64.9$	mph
	O	
Space mean speed for all vehicles,	$S = 53.9$	mph

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Merge Analysis

Analyst: NBS (Alt-9&4567_Ex7_NB-On_AM)
Agency/Co.: VHB
Date performed: 3/20/2013
Analysis time period: AM Peak Hour
Freeway/Dir of Travel: I-293 / NB
Junction: Exit 7 NB On Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt 9 & 4,5,6&7
Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	1185	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	305	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	570	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	2400	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1185	305	570	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	329	85	158	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1350	342	640	pcph

Estimation of V12 Merge Areas

$$L = 145.49 \quad (\text{Equation 25-2 or 25-3})$$

$$EQ$$

$$P = 0.600 \quad \text{Using Equation 1}$$

$$FM$$

$$v_{12} = v_{F \quad FM} (P) = 810 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v FO	1692	6900	No
v R12	1152	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 9.3 \quad \text{pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence A

Speed Estimation

Intermediate speed variable,	M = 0.277	
	S	
Space mean speed in ramp influence area,	S = 55.0	mph
	R	
Space mean speed in outer lanes,	S = 59.9	mph
	O	
Space mean speed for all vehicles,	S = 56.5	mph

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Merge Analysis

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 Agency/Co.: VHB
 Date performed: 3/20/2013
 Analysis time period: PM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 7 NB On Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt 9 & 4,5,6&7
 Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2800	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	360	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1000	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	2400	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2800	360	1000	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	778	100	278	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3189	404	1122	pcph

Estimation of V12 Merge Areas

L = 552.30 (Equation 25-2 or 25-3)
EQ
P = 0.600 Using Equation 1
FM
 $v_{12} = v_F (P_{FM}) = 1913 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	3593	6900	No
v _{R12}	2317	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 18.3 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	M = 0.305	
Space mean speed in ramp influence area,	S = 54.5	mph
Space mean speed in outer lanes,	S = 57.2	mph
Space mean speed for all vehicles,	S = 55.4	mph

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Merge Analysis

Analyst: NBS (Alt-10&4_Ex7_NB-On_AM)
 Agency/Co.: VHB
 Date performed: 3/20/2013
 Analysis time period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Junction: Exit 7 NB On Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt 10 & 4
 Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	1075	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	565	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	590	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	2400	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1075	565	590	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	299	157	164	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1224	634	662	pcph

Estimation of V12 Merge Areas

$$L = 181.01 \quad (\text{Equation 25-2 or 25-3})$$

$$EQ$$

$$P = 0.600 \quad \text{Using Equation 1}$$

$$FM$$

$$v_{12} = v_F (P_{FM}) = 734 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	1858	6900	No
v _{R12}	1368	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 10.8 \quad \text{pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	M = 0.280	
Space mean speed in ramp influence area,	S = 55.0	mph
Space mean speed in outer lanes,	S = 60.0	mph
Space mean speed for all vehicles,	S = 56.2	mph

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Merge Analysis

Analyst: NBS (Alt-10&4_Ex7_NB-On_PM)
Agency/Co.: VHB
Date performed: 3/20/2013
Analysis time period: PM Peak Hour
Freeway/Dir of Travel: I-293 / NB
Junction: Exit 7 NB On Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt 10 & 4
Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2685	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	575	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1055	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	2400	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2685	575	1055	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	746	160	293	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3058	645	1184	pcph

Estimation of V12 Merge Areas

L = 575.84 (Equation 25-2 or 25-3)
EQ
P = 0.600 Using Equation 1
FM
 $v_{12} = v_{F} (P_{FM}) = 1834 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	3703	6900	No
v _{R12}	2479	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 19.5 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	M = 0.312	
Space mean speed in ramp influence area,	S _R = 54.4	mph
Space mean speed in outer lanes,	S ₀ = 57.4	mph
Space mean speed for all vehicles,	S = 55.3	mph

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Diverge Analysis

Analyst: NBS (Alt-9&4_Ex7_SB-Off_AM)
Agency/Co.: VHB
Date performed: 3/20/2013
Analysis time period: AM Peak Hour
Freeway/Dir of Travel: I-293 / SB
Junction: Exit 7 SB Off Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt 9 & 4
Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2590	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	210	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	625	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	1200	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2590	210	625	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	719	58	174	v
Trucks and buses	4	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.980	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2935	236	701	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.676 \quad \text{Using Equation 5}$$

$$FD$$

$$v_{12} = v_R + (v_F - v_R) P = 2060 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{12} = v_{12}$	2935	6900	No
v_{12}	2060	4400	No
$v_{12} = v_F - v_R$	2699	6900	No
v_R	236	2000	No

Level of Service Determination (if not F)

$$\text{Density, } D = 4.252 + 0.0086 v_{12} - 0.009 L = 17.6 \quad \text{pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	D = 0.449	
Space mean speed in ramp influence area,	S _R = 51.9	mph
Space mean speed in outer lanes,	S ₀ = 65.8	mph
Space mean speed for all vehicles,	S = 55.4	mph

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Diverge Analysis

Analyst: NBS (Alt-9&4_Ex7_SB-Off_PM)
Agency/Co.: VHB
Date performed: 3/20/2013
Analysis time period: PM Peak Hour
Freeway/Dir of Travel: I-293 / SB
Junction: Exit 7 SB Off Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt 9 & 4
Description: FEET I-293

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	3		
Free-flow speed on freeway	60.0	mph	
Volume on freeway	2310	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	275	vph	
Length of first accel/decel lane	486	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent ramp	350	vph	
Position of adjacent ramp	Downstream		
Type of adjacent ramp	On		
Distance to adjacent ramp	1200	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2310	275	350	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	642	76	97	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2631	309	393	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.688 \quad \text{Using Equation 5}$$

$$FD$$

$$v_{12} = v_R + (v_F - v_R) P = 1888 \quad pc/h$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2631	6900	No
v_{12}	1888	4400	No
$v_{FO} = v_F - v_R$	2322	6900	No
v_R	309	2000	No

Level of Service Determination (if not F)

$$\text{Density, } D = 4.252 + 0.0086 \frac{v_{12}}{R} - 0.009 \frac{L}{D} = 16.1 \quad pc/mi/ln$$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	$D = 0.456$	
	S	
Space mean speed in ramp influence area,	$S = 51.8$	mph
	R	
Space mean speed in outer lanes,	$S = 65.8$	mph
	O	
Space mean speed for all vehicles,	$S = 55.1$	mph

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Diverge Analysis

Analyst: NBS (Alt-10&4_Ex7_SB-Off_AM)
 Agency/Co.: VHB
 Date performed: 3/20/2013
 Analysis time period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / SB
 Junction: Exit 7 SB Off Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt 10 & 4
 Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2690	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	410	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	675	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	1200	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2690	410	675	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	747	114	188	v
Trucks and buses	4	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.980	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3049	460	758	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.663 \quad \text{Using Equation 5}$$

$$v_{12} = v_R + (v_F - v_R) P = 2176 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	3049	6900	No
v_{12}	2176	4400	No
$v_{FO} = v_F - v_R$	2589	6900	No
v_R	460	2000	No

Level of Service Determination (if not F)

$$\text{Density, } D = 4.252 + 0.0086 v_{12} - 0.009 L = 18.6 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	$D = 0.469$	
Space mean speed in ramp influence area,	$S_R = 51.6$	mph
Space mean speed in outer lanes,	$S_0 = 65.8$	mph
Space mean speed for all vehicles,	$S = 55.0$	mph

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Diverge Analysis

Analyst: NBS (Alt-10&4_Ex7_SB-Off_PM)
 Agency/Co.: VHB
 Date performed: 3/20/2013
 Analysis time period: PM Peak Hour
 Freeway/Dir of Travel: I-293 / SB
 Junction: Exit 7 SB Off Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt 10 & 4
 Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2460	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	535	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	375	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	1200	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2460	535	375	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	683	149	104	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2802	600	421	pcph

Estimation of V12 Diverge Areas

$$L = \text{EQ} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.662 \quad \text{Using Equation 5}$$

$$v_{12} = v_R + (v_F - v_R) P = 2058 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2802	6900	No
v_{12}	2058	4400	No
$v_{FO} = v_F - v_R$	2202	6900	No
v_R	600	2000	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 17.6 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	$D = 0.482$	
Space mean speed in ramp influence area,	$S_R = 51.3$	mph
Space mean speed in outer lanes,	$S_0 = 65.8$	mph
Space mean speed for all vehicles,	$S = 54.5$	mph

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Diverge Analysis

Analyst: NBS (Alt-9&567_Ex7_SB-Off_AM)
Agency/Co.: VHB
Date performed: 3/20/2013
Analysis time period: AM Peak Hour
Freeway/Dir of Travel: I-293 / SB
Junction: Exit 7 SB Off Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt 9 & 5,6&7
Description: FEET I-293

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	3		
Free-flow speed on freeway	60.0	mph	
Volume on freeway	2590	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	210	vph	
Length of first accel/decel lane	486	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent ramp	625	vph	
Position of adjacent ramp	Downstream		
Type of adjacent ramp	On		
Distance to adjacent ramp	1200	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2590	210	625	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	719	58	174	v
Trucks and buses	4	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.980	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2935	236	701	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.676 \quad \text{Using Equation 5}$$

$$FD$$

$$v_{12R} = v_F + (v_R - v_F) P = 2060 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{12R} = v_F$	2935	6900	No
v_{12R}	2060	4400	No
$v_{12R} = v_R - v_F$	2699	6900	No
v_R	236	2000	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12R} - 0.009 L_D = 17.6 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	$D = 0.449$	
Space mean speed in ramp influence area,	$S_R = 51.9$	mph
Space mean speed in outer lanes,	$S_0 = 65.8$	mph
Space mean speed for all vehicles,	$S = 55.4$	mph

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Diverge Analysis

Analyst: NBS (Alt-9&567_Ex7_SB-Off_PM)
Agency/Co.: VHB
Date performed: 3/20/2013
Analysis time period: PM Peak Hour
Freeway/Dir of Travel: I-293 / SB
Junction: Exit 7 SB Off Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt 9 & 5,6&7
Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2330	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	275	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	350	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	1200	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2330	275	350	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	647	76	97	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2654	309	393	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)
EQ
P = 0.679 Using Equation 5
FD
 $v_{12} = v_R + (v_F - v_R) P = 1902 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2654	6900	No
v_{12}	1902	4400	No
$v_{FO} = v_F - v_R$	2345	6900	No
v_R	309	2000	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 16.2 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	D = 0.456	
Space mean speed in ramp influence area,	S = 51.8	mph
Space mean speed in outer lanes,	S = 65.8	mph
Space mean speed for all vehicles,	S = 55.1	mph

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Merge Analysis

Analyst: NBS (Alt-9&4_Ex7_SB-On_AM)
 Agency/Co.: VHB
 Date performed: 3/20/2013
 Analysis time period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / SB
 Junction: Exit 7 SB On Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build Alt 9&4
 Description: FEET I-293

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	60.0	mph	
Volume on freeway	2380	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	625	vph	
Length of first accel/decel lane	800	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	210	vph	
Position of adjacent Ramp	Upstream		
Type of adjacent Ramp	Off		
Distance to adjacent Ramp	1200	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2380	625	210	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	661	174	58	v
Trucks and buses	4	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.980	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2697	701	236	pcph

Estimation of V12 Merge Areas

L = 510.57 (Equation 25-2 or 25-3)
EQ
P = 0.600 Using Equation 1
FM
 $v_{12} = v_{F \text{ FM}} = 1618 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v FO	3398	6900	No
v R12	2319	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 18.2 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	M = 0.305	
Space mean speed in ramp influence area,	S = 54.5	mph
Space mean speed in outer lanes,	S = 57.9	mph
Space mean speed for all vehicles,	S = 55.6	mph

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Merge Analysis

Analyst: NBS (Alt-9&4_Ex7_SB-On_PM)
Agency/Co.: VHB
Date performed: 3/20/2013
Analysis time period: PM Peak Hour
Freeway/Dir of Travel: I-293 / SB
Junction: Exit 7 SB On Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build Alt 9&4
Description: FEET I-293

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	60.0	mph	
Volume on freeway	2035	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	350	vph	
Length of first accel/decel lane	800	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	275	vph	
Position of adjacent Ramp	Upstream		
Type of adjacent Ramp	Off		
Distance to adjacent Ramp	1200	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2035	350	275	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	565	97	76	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2318	393	309	pcph

Estimation of V12 Merge Areas

L = 363.55 (Equation 25-2 or 25-3)
EQ
P = 0.600 Using Equation 1
FM
 $v_{12} = v_{F} (P_{FM}) = 1391 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v FO	2711	6900	No
v R12	1784	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 14.2 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, M = 0.288
Space mean speed in ramp influence area, S = 54.8 mph
Space mean speed in outer lanes, S = 58.5 mph
Space mean speed for all vehicles, S = 56.0 mph

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Merge Analysis

Analyst: NBS (Alt-10&4_Ex7_SB-On_AM)
 Agency/Co.: VHB
 Date performed: 3/20/2013
 Analysis time period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / SB
 Junction: Exit 7 SB On Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build Alt 10&4
 Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2280	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	675	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	410	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1200	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2280	675	410	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	633	188	114	v
Trucks and buses	4	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.980	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2584	758	460	pcph

Estimation of V12 Merge Areas

L = 498.59 (Equation 25-2 or 25-3)
EQ
P = 0.600 Using Equation 1
FM
 $v_{12} = v_F (P_{FM}) = 1550 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v FO	3342	6900	No
v R12	2308	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 18.1 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, M = 0.304
Space mean speed in ramp influence area, S_R = 54.5 mph
Space mean speed in outer lanes, S₀ = 58.1 mph
Space mean speed for all vehicles, S = 55.6 mph

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Merge Analysis

Analyst: NBS (Alt-10&4_Ex7_SB-On_PM)
 Agency/Co.: VHB
 Date performed: 3/20/2013
 Analysis time period: PM Peak Hour
 Freeway/Dir of Travel: I-293 / SB
 Junction: Exit 7 SB On Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build Alt 10&4
 Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	1925	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	375	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	535	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1200	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1925	375	535	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	535	104	149	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2192	421	600	pcph

Estimation of V12 Merge Areas

L = 342.58 (Equation 25-2 or 25-3)
EQ
P = 0.600 Using Equation 1
FM
 $v_{12} = v_{F} (P_{FM}) = 1315 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v FO	2613	6900	No
v R12	1736	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 13.8 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, M = 0.287
Space mean speed in ramp influence area, S = 54.8 mph
Space mean speed in outer lanes, S = 58.6 mph
Space mean speed for all vehicles, S = 56.1 mph

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Merge Analysis

Analyst: NBS (Alt-9&567_Ex7_SB-On_AM)
 Agency/Co.: VHB
 Date performed: 3/20/2013
 Analysis time period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / SB
 Junction: Exit 7 SB On Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt 9 & 5,6,7
 Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2380	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	625	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	210	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1200	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2380	625	210	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	661	174	58	v
Trucks and buses	4	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.980	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2697	701	236	pcph

Estimation of V12 Merge Areas

L = 510.57 (Equation 25-2 or 25-3)
EQ
P = 0.600 Using Equation 1
FM
 $v_{12} = v_{F} (P_{FM}) = 1618 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	3398	6900	No
v _{R12}	2319	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 18.2 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, M = 0.305
S
Space mean speed in ramp influence area, S = 54.5 mph
R
Space mean speed in outer lanes, S = 57.9 mph
O
Space mean speed for all vehicles, S = 55.6 mph

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Merge Analysis

Analyst: NBS (Alt-9&567_Ex7_SB-On_PM)
 Agency/Co.: VHB
 Date performed: 3/20/2013
 Analysis time period: PM Peak Hour
 Freeway/Dir of Travel: I-293 / SB
 Junction: Exit 7 SB On Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt 9 & 5,6,7
 Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2055	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	350	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	275	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1200	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2055	350	275	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	571	97	76	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2340	393	309	pcph

Estimation of V12 Merge Areas

L = 368.26 (Equation 25-2 or 25-3)
EQ
P = 0.600 Using Equation 1
FM
 $v_{12} = v_F (P_{FM}) = 1404$ pc/h

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	2733	6900	No
v _{R12}	1797	4600	No

Level of Service Determination (if not F)

Density, $D_R = 5.475 + 0.00734 v_{R12} + 0.0078 v_A - 0.00627 L$ = 14.3 pc/mi/ln
Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, M = 0.289
Space mean speed in ramp influence area, S_R = 54.8 mph
Space mean speed in outer lanes, S₀ = 58.4 mph
Space mean speed for all vehicles, S = 56.0 mph

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Merge Analysis

Analyst: NBS (Alt-8&4_Ex7_NB-On_AM)
Agency/Co.: VHB
Date performed: 3/20/2013
Analysis time period: AM Peak Hour
Freeway/Dir of Travel: I-293 / NB
Junction: Exit 7 NB On Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt 8&4
Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	935	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	155	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	970	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	2300	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	935	155	970	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	260	43	269	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1065	174	1089	pcph

Estimation of V12 Merge Areas

L = 48.55 (Equation 25-2 or 25-3)
EQ
P = 0.600 Using Equation 1
FM
 $v_{12} = v_{F} (P_{FM}) = 639$ pc/h

Capacity Checks

	Actual	Maximum	LOS F?
v FO	1239	6900	No
v R12	813	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 6.7$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence A

Speed Estimation

Intermediate speed variable,	M	= 0.274	
	S		
Space mean speed in ramp influence area,	S	= 55.1	mph
	R		
Space mean speed in outer lanes,	S	= 60.0	mph
	O		
Space mean speed for all vehicles,	S	= 56.7	mph

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Merge Analysis

Analyst: NBS (Alt-8&4_Ex7_NB-On_PM)
Agency/Co.: VHB
Date performed: 3/20/2013
Analysis time period: PM Peak Hour
Freeway/Dir of Travel: I-293 / NB
Junction: Exit 7 NB On Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt 8&4
Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2700	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	160	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1300	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	2300	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2700	160	1300	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	750	44	361	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3075	180	1459	pcph

Estimation of V12 Merge Areas

L = 479.97 (Equation 25-2 or 25-3)
EQ
P = 0.600 Using Equation 1
FM
 $v_{12} = v_{F} (P_{FM}) = 1845 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v	3255	6900	No
FO			
v	2025	4600	No
R12			

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 16.2 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	M = 0.295	
	S	
Space mean speed in ramp influence area,	S = 54.7	mph
	R	
Space mean speed in outer lanes,	S = 57.4	mph
	O	
Space mean speed for all vehicles,	S = 55.7	mph

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Diverge Analysis

Analyst: NBS (Alt-8&4_Ex7_SB-Off_AM)
Agency/Co.: VHB
Date performed: 3/20/2013
Analysis time period: AM Peak Hour
Freeway/Dir of Travel: I-293 / SB
Junction: Exit 7 SB Off Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build - Alt 8 & 4
Description: FEET I-293

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	2240	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	260	vph
Length of first accel/decel lane	486	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	1025	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	2000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2240	260	1025	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	622	72	285	v
Trucks and buses	4	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.980	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2539	292	1150	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)
EQ
P = 0.683 Using Equation 5
FD
 $v_{12} = v_R + (v_F - v_R) P = 1827 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{12} = v_F$	2539	6900	No
v_{12}	1827	4400	No
$v_{12} = v_F - v_R$	2247	6900	No
v_R	292	2000	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 15.6 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	D = 0.454	
Space mean speed in ramp influence area,	S _R = 51.8	mph
Space mean speed in outer lanes,	S ₀ = 65.8	mph
Space mean speed for all vehicles,	S = 55.1	mph

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Diverge Analysis

Analyst: NBS (Alt-8&4_Ex7_SB-Off_PM)
 Agency/Co.: VHB
 Date performed: 3/20/2013
 Analysis time period: PM Peak Hour
 Freeway/Dir of Travel: I-293 / SB
 Junction: Exit 7 SB Off Ramp
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build - Alt 8 & 4
 Description: FEET I-293

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	3		
Free-flow speed on freeway	60.0	mph	
Volume on freeway	2120	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	375	vph	
Length of first accel/decel lane	486	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent ramp	650	vph	
Position of adjacent ramp	Downstream		
Type of adjacent ramp	On		
Distance to adjacent ramp	2000	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2120	375	650	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	589	104	181	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2414	421	729	pcph.

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.680 \quad \text{Using Equation 5}$$

$$v_{12} = v_R + (v_F - v_R) P = 1777 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{12} = v_{Fi}$	2414	6900	No
v_{12}	1777	4400	No
$v_{FO} = v_F - v_R$	1993	6900	No
v_R	421	2000	No

Level of Service Determination (if not F)

$$\text{Density, } D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 15.2 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	$D = 0.466$	
Space mean speed in ramp influence area,	$S_R = 51.6$	mph
Space mean speed in outer lanes,	$S_0 = 65.8$	mph
Space mean speed for all vehicles,	$S = 54.7$	mph

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Merge Analysis

Analyst: NBS (Alt-8&4_Ex7_SB-On_PM)
Agency/Co.: VHB
Date performed: 3/20/2013
Analysis time period: PM Peak Hour
Freeway/Dir of Travel: I-293 / SB
Junction: Exit 7 SB On Ramp
Jurisdiction: NHDOT
Analysis Year: 2035 Build Alt 8&4
Description: FEET I-293

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	60.0	mph
Volume on freeway	1745	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	650	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	540	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1900	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1745	650	540	vph
Peak-hour factor, PHF	0.90	0.90	0.90	
Peak 15-min volume, v15	485	181	150	v
Trucks and buses	5	2	2	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.976	0.990	0.990	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1987	729	606	pcph

Estimation of V12 Merge Areas

$$L = 3104.51 \quad (\text{Equation 25-2 or 25-3})$$

EQ

$$P = 0.633 \quad \text{Using Equation 3}$$

FM

$$v_{12} = v_{F \text{ FM}} = 1257 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	2716	6900	No
v _{R12}	1986	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 15.6 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	M = 0.293	
	S	
Space mean speed in ramp influence area,	S = 54.7	mph
	R	
Space mean speed in outer lanes,	S = 59.2	mph
	O	
Space mean speed for all vehicles,	S = 55.8	mph

HCS2000: Freeway Weaving Release 4.1f

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Operational Analysis

Analyst: NBS (A_Alt-4&8_Ex6-7_NB_AM)
 Agency/Co.: VHB
 Date Performed: 3/20/2013
 Analysis Time Period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / NB
 Weaving Location: Exit 6 to Exit 7
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build Alt 4 & 8
 Description: FEET I-293

Inputs

Freeway free-flow speed, SFF	60	mph
Weaving number of lanes, N	4	
Weaving segment length, L	1550	ft
Terrain type	Level	
Grade		%
Length		mi
Weaving type	A	
Volume ratio, VR	0.84	
Weaving ratio, R	0.46	

Conversion to pc/h Under Base Conditions

	Non-Weaving		Weaving		
	V A-C	V B-D	V A-D	V B-C	
Volume, V	200	100	870	735	veh/h
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	
Peak 15-min volume, v15	56	28	242	204	v
Trucks and buses	5	2	2	2	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.976	0.990	0.990	0.990	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	227	112	976	824	pc/h

Weaving and Non-Weaving Speeds

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	1.67	0.33
Weaving and non-weaving speeds, Si	33.76	52.66
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7)	2.85
Maximum number of lanes, Nw (max) (Exhibit 24-7)	1.40
Type of operation is	Constrained

 Weaving Segment Speed, Density, Level of Service and Capacity

Weaving segment speed, S	35.80	mph
Weaving segment density, D	14.94	pc/mi/ln
Level of service, LOS	B	
Capacity of base condition, cb	6843	pc/h
Capacity as a 15-minute flow rate, c	6676	pc/h
Capacity as a full-hour volume, ch	6008	pc/h

 Limitations on Weaving Segments

	Analyzed	If Max Exceeded	See Note
Weaving flow rate, Vw	1800	2800	a
Average flow rate (pcphpl)	534	2300	b
Volume ratio, VR	0.84	0.35	c
Weaving ratio, R	0.46	N/A	d
Weaving length (ft)	1550	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

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Operational Analysis

Analyst: NBS (A_Alt-4&8_Ex6-7_NB_PM)
Agency/Co.: VHB
Date Performed: 3/20/2013
Analysis Time Period: PM Peak Hour
Freeway/Dir of Travel: I-293 / NB
Weaving Location: Exit 6 to Exit 7
Jurisdiction: NHDOT
Analysis Year: 2035 Build Alt 4 & 8
Description: FEET I-293

Inputs

Freeway free-flow speed, SFF	60	mph
Weaving number of lanes, N	4	
Weaving segment length, L	1550	ft
Terrain type	Level	
Grade		%
Length		mi
Weaving type	A	
Volume ratio, VR	0.61	
Weaving ratio, R	0.49	

Conversion to pc/h Under Base Conditions

	Non-Weaving		Weaving		
	V	V	V	V	
	A-C	B-D	A-D	B-C	
Volume, V	1450	100	1200	1250	veh/h
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	
Peak 15-min volume, v15	403	28	333	347	v
Trucks and buses	5	2	2	2	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.976	0.990	0.990	0.990	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	1651	112	1346	1402	pc/h

Weaving and Non-Weaving Speeds

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	2.55	0.50
Weaving and non-weaving speeds, Si	29.07	48.24
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7)	2.50
Maximum number of lanes, Nw (max) (Exhibit 24-7)	1.40
Type of operation is	Constrained

 Weaving Segment Speed, Density, Level of Service and Capacity

Weaving segment speed, S	34.42	mph
Weaving segment density, D	32.77	pc/mi/ln
Level of service, LOS	D	
Capacity of base condition, cb	6843	pc/h
Capacity as a 15-minute flow rate, c	6676	pc/h
Capacity as a full-hour volume, ch	6008	pc/h

 Limitations on Weaving Segments

	Analyzed	If Max Exceeded	See Note
		Maximum	Note
Weaving flow rate, Vw	2748	2800	a
Average flow rate (pcphpl)	1127	2300	b
Volume ratio, VR	0.61	0.35	c
Weaving ratio, R	0.49	N/A	d
Weaving length (ft)	1550	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

HCS2000: Freeway Weaving Release 4.1f

VHB, Inc
 VHB
 101 Walnut St.
 Watertown MA

Phone: 617-924-1770
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Operational Analysis

Analyst: NBS (A_Alt-4&8_Ex6-7_SB_AM)
 Agency/Co.: VHB
 Date Performed: 3/20/2013
 Analysis Time Period: AM Peak Hour
 Freeway/Dir of Travel: I-293 / SB
 Weaving Location: Exit 6 to Exit 7
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build Alt 4 & 8
 Description: FEET I-293

Inputs

Freeway free-flow speed, SFF	60	mph
Weaving number of lanes, N	4	
Weaving segment length, L	1600	ft
Terrain type	Level	
Grade		%
Length		mi
Weaving type	A	
Volume ratio, VR	0.64	
Weaving ratio, R	0.48	

Conversion to pc/h Under Base Conditions

	Non-Weaving		Weaving		
	V A-C	V B-D	V A-D	V B-C	
Volume, V	980	100	1000	925	veh/h
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	
Peak 15-min volume, v15	272	28	278	257	v
Trucks and buses	4	2	2	2	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.980	0.990	0.990	0.990	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	1110	112	1122	1038	pc/h

Weaving and Non-Weaving Speeds

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	1.96	0.36
Weaving and non-weaving speeds, Si	31.90	51.66
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7)	2.50
Maximum number of lanes, Nw (max) (Exhibit 24-7)	1.40
Type of operation is	Constrained

Weaving Segment Speed, Density, Level of Service and Capacity

Weaving segment speed, S	37.01	mph
Weaving segment density, D	22.84	pc/mi/ln
Level of service, LOS	C	
Capacity of base condition, cb	6876	pc/h
Capacity as a 15-minute flow rate, c	6741	pc/h
Capacity as a full-hour volume, ch	6067	pc/h

Limitations on Weaving Segments

	Analyzed	If Max Exceeded	See Note
Weaving flow rate, Vw	2160	2800	a
Average flow rate (pcphpl)	845	2300	b
Volume ratio, VR	0.64	0.35	c
Weaving ratio, R	0.48	N/A	d
Weaving length (ft)	1600	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

HCS2000: Freeway Weaving Release 4.1f

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Operational Analysis

Analyst: NBS (A_Alt-4&8_Ex6-7_SB_PM)
 Agency/Co.: VHB
 Date Performed: 3/20/2013
 Analysis Time Period: PM Peak Hour
 Freeway/Dir of Travel: I-293 / SB
 Weaving Location: Exit 6 to Exit 7
 Jurisdiction: NHDOT
 Analysis Year: 2035 Build Alt 4 & 8
 Description: FEET I-293

Inputs

Freeway free-flow speed, SFF	60	mph
Weaving number of lanes, N	4	
Weaving segment length, L	1600	ft
Terrain type	Level	
Grade		%
Length		mi
Weaving type	A	
Volume ratio, VR	0.45	
Weaving ratio, R	0.45	

Conversion to pc/h Under Base Conditions

	Non-Weaving		Weaving		
	V A-C	V B-D	V A-D	V B-C	
Volume, V	1255	50	490	600	veh/h
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	
Peak 15-min volume, v15	349	14	136	167	v
Trucks and buses	5	2	2	2	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.976	0.990	0.990	0.990	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	1429	56	549	673	pc/h

Weaving and Non-Weaving Speeds

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	1.21	0.17
Weaving and non-weaving speeds, Si	37.64	57.82
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7)	1.94
Maximum number of lanes, Nw (max) (Exhibit 24-7)	1.40
Type of operation is	Constrained

 Weaving Segment Speed, Density, Level of Service and Capacity

Weaving segment speed, S	46.55	mph
Weaving segment density, D	14.54	pc/mi/ln
Level of service, LOS	B	
Capacity of base condition, cb	6876	pc/h
Capacity as a 15-minute flow rate, c	6708	pc/h
Capacity as a full-hour volume, ch	6037	pc/h

 Limitations on Weaving Segments

	Analyzed	If Max Exceeded	See Note
		Maximum	Note
Weaving flow rate, Vw	1222	2800	a
Average flow rate (pcphpl)	676	2300	b
Volume ratio, VR	0.45	0.35	c
Weaving ratio, R	0.45	N/A	d
Weaving length (ft)	1600	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.